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Total number of printed pages – 2

B. Tech  
PECI 5301

**Sixth Semester Examination – 2013**

**DESIGN OF STEEL STRUCTURES**

**BRANCH : CIVIL**

**QUESTION CODE : A 193**

**Full Marks – 70**

**Time : 3 Hours**

*Answer Question No. 1 which is compulsory and any **five** from the rest.*

*The figures in the right-hand margin indicate marks.*

*Use of relevant IS codes and steel tables are permitted.*

*Assume suitable data wherever required.*

1. Answer the following questions very shortly :

2×10

- Alloy steel
- Compact section
- Residual stress
- Serviceability
- Design wind speed
- Block shear
- Slenderness ratio
- Moment resisting frame
- Bearing stiffener
- Butt weld



2. Determine the tension capacity of a 125 × 75 × 6 angle of Fe 410 steel assuming the connection is through the longer leg with 3 numbers M20 bolt. Will there be any difference if the shorter leg is connected ?

10

**P.T.O.**

3. Design a single angle strut carrying a factored compressive load of 65 kN with length between centre to centre of intersection as 3.0 m. Also design the bolted end connection. 10
4. Design a battened column with effective length of 5.0 m with two channels back to back to carry a factored load of 2000 kN. 10
5. A beam is to carry a uniformly distributed dead load of 300 kN (total) and superimposed load of 40 kN/m. The beam is simply supported over a clear span of 8.0 m and rests over stiff bearings of 215 mm at the ends. Design the beam assuming full lateral support for the compression flange. 10
6. Compute the flexural strength of a plate girder which consists of a 8 × 1200 mm web and 30 × 450 mm flanges. Use Fe410 steel and assume the compression flange to be laterally supported. 10
7. Design a double cover butt joint to join two plates of size 200 × 10 mm of Fe410 to mobilize full tensile strength using shop welding. 10
8. Determine the maximum load inclined at 45° to the horizontal that can be transmitted if 5-16 mm diameter grade 6.8 bolts are used and the plates are 10 mm thick. All dimensions are in mm. 10

